# TRAFFIC SIGNAL **WARRANT ANALYSIS**

Intersection of:

Northwest Highway at Tulane Boulevard In Dallas, Texas

Prepared for:

City of Dallas

On behalf of:

**Preston Place Condominium Association** 

Prepared by:

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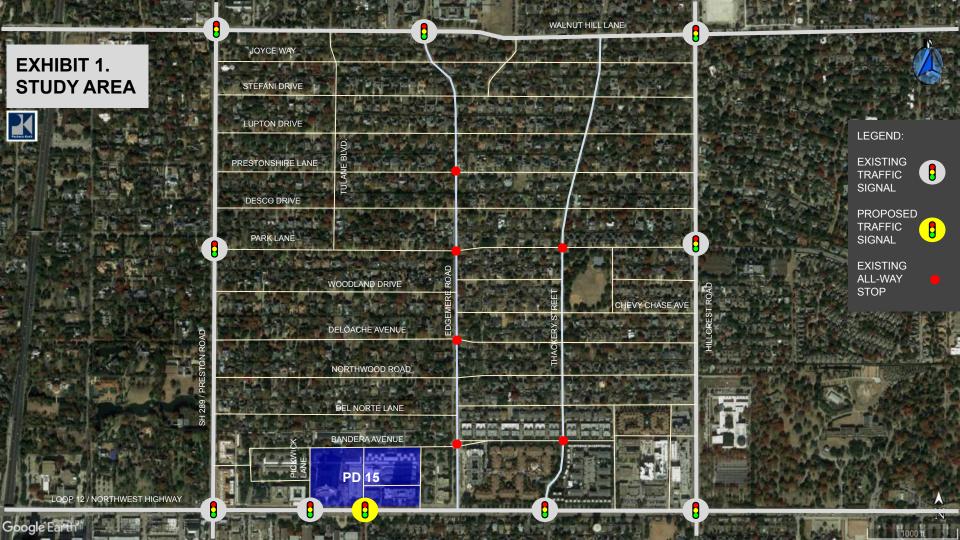
## **EXECUTIVE SUMMARY**

The services of **Pacheco Koch** (PK) were retained by **Preston Place Condominium Association** to conduct a cursory Traffic Signal Warrant Analysis (TSWA) for the proposed, new intersection of Northwest Highway at Tulane Boulevard (a private street) in Dallas, Texas. The analysis was based upon existing conditions, as collected on Wednesday, March 27, 2019, with the addition of projected increases in site-generated traffic resulting from potential redevelopment within PD 15. This engineering study was performed in accordance with the traffic signal warrant criteria described in the latest edition of the Texas Manual on Uniform Traffic Control Devices.

Based upon Pacheco Koch's technical analysis of the traffic-volume-related traffic signal warrant criteria, it was determined that, at buildout conditions of the potential redevelopment of PD 15, the subject intersection will NOT meet the minimum warrants for the installation of a traffic signal.

The findings of this analysis are subject to the review by and approval of the City of Dallas staff.

END





## TRAFFIC SIGNAL WARRANT ANALYSIS

## Intersection of Northwest Highway at Tulane Boulevard

Dallas, Texas

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## INTRODUCTION

The services of **Pacheco Koch** (PK) were retained by **Preston Place Condominium Association** to conduct a Traffic Signal Warrant Analysis (TSWA) for the *proposed*, new intersection of Northwest Highway at Tulane Boulevard in Dallas, Texas. This analysis was based upon projected traffic volumes, derived from existing traffic volumes collected at nearby intersections on Wednesday, March 27, 2019, with the addition of projected increases in site-generated traffic resulting from potential redevelopment within PD 15. A location map of the subject intersection (**Exhibit 1**) is provided for reference following the **EXECUTIVE SUMMARY**.

For a traffic signal to be warranted, an engineering study of the intersection is required to determine if at least one of nine, predefined, traffic signal warrants are (or will be) satisfied. The warrant criteria are summarized in the 2014 Texas Manual on Uniform Traffic Control Devices (TMUTCD). To obtain approval for installation of a traffic signal, staff of the agency responsible for traffic operations of the intersection must agree that one or more traffic signal warrants is satisfied and determine that installation of a traffic signal is appropriate and can be operated safely and efficiently.

This TSWA was prepared by traffic engineers at Pacheco Koch (the "Engineer") in accordance with industry and local standards. Pacheco Koch is a licensed engineering firm based in Dallas, Texas, that provides professional engineering and related services.

# **BACKGROUND**

## Description of Intersection

Currently, Tulane Boulevard (a private street) terminates at Northwest Parkway and does not intersect Northwest Highway. However, creation of a new intersection with Northwest Highway has been proposed in order to provide direct access to the PD 15 district. This hypothetical intersection is the subject of this analysis.

The major street, Northwest Highway, has a six-lane, median-divided cross-section with a 35-MPH posted speed limit. [NOTE: The City of Dallas staff has indicated that an increase to a 45-MPH speed limit has been approved and will be implemented in the future.] Tulane Boulevard, north of Northwest Parkway, has a narrow cross-section of approximately 20 feet.

Nearby traffic signals are located at the intersection of Northwest Highway at Pickwick Lane approximately 600-feet west of the subject intersection and at the intersection of Northwest Highway and Thackery Street approximately 2,000-feet east of the subject intersection.



## **Existing Traffic Volumes**

Data used in this analysis were derived from traffic counts taken at nearby intersections on Wednesday, March 27, 2019. (Pneumatic 'road tube' counts were used for off-peak period volumes). Detailed volume data are summarized in **Appendix A**.

#### Site-Generated Traffic Volumes

For this analysis, projected traffic volumes generated by the potential PD 15 redevelopment, as described in the associated traffic study prepared by Pacheco Koch, were added to the background traffic volumes. The site generated volumes were calculated by the typical four-step of trip generation; trip distribution; traffic assignment; and mode split. **Appendix B** provides summaries of these analyses.

## Projected Traffic Volumes

The traffic volumes used in the traffic signal warrant analysis represent the sum of projected background traffic volumes plus projected PD 15 redevelopment traffic volumes. (NOTE: Existing traffic volumes were adjusted to account for anticipated changes in traffic patterns resulting from the hypothetical installation of a traffic signal at the Northwest Highway-Tulane intersection.)

## TRAFFIC SIGNAL WARRANT ANALYSIS

## Traffic Signal Warrants

The TMUTCD defines a series of nine traffic signal warrants to be used in the investigation of a traffic signal installation. These warrants are listed as follows:

Warrant 1 — Eight-Hour Vehicular Volume

Warrant 2 — Four-Hour Vehicular Volume

Warrant 3 — Peak Hour

Warrant 4 — Pedestrian Volume

Warrant 5 — School Crossing

Warrant 6 — Coordinated Signal System

Warrant 7 — Crash Experience

Warrant 8 — Roadway Network

Warrant 9 — Intersection Near a Grade Crossing

A description of the warrants as obtained from §4C-01 of the TMUTCD are provided in **Appendix C**. It is important to note that the intersection Level of Service (LOS) is not a warrant for signal installation. The TMUTCD states:

The satisfaction of a traffic signal warrant or warrants shall not in itself require the installation of a traffic control signal. A traffic control signal should not be installed unless one or more of the factors described in this [manual] are met. A traffic control signal should not be installed unless an engineering study indicates that installing a traffic control signal will improve the overall safety and/or operation of the intersection.



## Study Scope

The scope of this Traffic Signal Warrant Analysis was to evaluate whether the projected traffic volumes will be sufficient to warrant installation of a traffic signal. Therefore, Warrant 1: Eight-Hour Vehicular Volumes, Warrant 2: Four-Hour Vehicular Volumes, and Warrant 3: Peak Hour were analyzed. (NOTE: Some agencies reserve application of Warrant 3 for special cases only. However, analysis results for Warrant 3 are provided for consideration.)

Based upon the location and physical conditions of the hypothetical intersection, it is Pacheco Koch's finding that Warrants 4-9 do not apply to the subject intersection.

## Analysis Results

By applying all the warrant criteria and data, the applicable traffic signal warrants were analyzed. A detailed summary of this traffic signal warrant analysis data are provided in **Appendix D**; and a written summary of each warrant is provided in the following sections.

#### Warrant 1 – Eight-Hour Vehicular Volumes

Condition A — This application is used where a large volume of intersecting traffic is the principal reason for installing a traffic signal. The total major street approach volumes must be greater than 600 vehicles each hour for a minimum of eight hours. The highest minor street approach volumes must be greater than 200 vehicles each hour for a minimum of eight hours.

❖ Finding: No (0) hours of both the major and minor street vehicular traffic volumes meet the warrant criteria; Condition A is NOT satisfied.

Condition B — This application is used where a large volume of major street traffic impedes the movement of the minor street traffic. The total major street approach volumes must be greater than 900 vehicles each hour for a minimum of eight hours. The highest minor street approach volumes must be greater than 100 vehicles each hour for a minimum of eight hours.

❖ Finding: No (0) hours of both the major and minor street vehicular traffic volumes meet the warrant criteria; Condition B is NOT satisfied.

Condition A and Condition B Combination (80% Factor) — This alternative warrant is only applicable if Condition A and Condition B are not individually satisfied.

❖ Finding: Two (2) hours of the Condition A-Condition B Combination meet the warrant criteria; the Combination warrant is NOT satisfied.

#### Warrant 2 - Four-Hour Vehicular Volumes

This warrant is for use in applications where the intersecting traffic is the main reason for installing a traffic signal. The total major street approach volumes, when plotted with the corresponding minor street volumes (from either approach), must fall above the applicable curve on Figure 4C-1 for at least four hours.

❖ Finding: No (0) hours of both the major and minor street vehicular traffic volumes meet the warrant criteria; Warrant 2 is NOT satisfied.



#### Warrant 3 - Peak Hour

This warrant is for use in applications where, for a minimum of one hour (i.e., any four, consecutive, 15-minute increments) of an average day, the minor street suffers excessive delay when entering or crossing the major street. The warrant is satisfied if the criteria in either of two categories are met:

- A. If all three of the following conditions exist for the same 1 hour:
  - The total stopped time delay experienced by the traffic on the minor-street approach (one direction only) controlled by a STOP sign equals or exceeds: 4 vehicle-hours for a one-lane approach, or 5 vehicles-hours for a two-lane approach; and
  - 2. The volume on the same minor-street approach equals or exceeds 100 vehicles per hour for one moving lane of traffic or 150 vehicles per hour for two moving lanes; and
  - 3. The total entering volume serviced during the hour equals or exceeds 650 vehicles per hour for intersections with three approach or 800 vehicles per hour for intersections with four or more approaches.
- Finding: (not analyzed)
- B. The total major street approach volumes, when plotted with the corresponding minor street volumes, must fall above the applicable curve on Figure 4C-3 for at least one hour.
- ❖ Finding: No (0) hours of both the major and minor street vehicular traffic volumes meet the peak hour warrant criteria; Warrant 3 is NOT satisfied.



## **SUMMARY OF FINDINGS**

NOTE: Recommendations presented in this report reflect the opinion of Pacheco Koch based solely upon technical analysis and professional judgment but are not intended to infer mandates or funding responsibility. Installation of a traffic signal, or any other improvements in the public right-of-way are subject to approval of the responsible agency(-ies). Legal precedents and other agency standards apply with regard to funding responsibilities.

The findings of this Traffic Signal Warrant Analysis, which include projected sitegenerated traffic volumes at site buildout conditions, are summarized in the following table:

Table 3. Summary of Results

TRAFFIC SIGNAL WARRANT	STATUS
Warrant 1. Eight-Hour Vehicular Volume	Not met
Warrant 2. Four-Hour Vehicular Volume	Not met
Warrant 3. Peak-Hour	Not met

Based upon Pacheco Koch's technical analysis of the traffic-volume-related traffic signal warrant criteria defined in the Texas Manual on Uniform Traffic Control Devices, it is projected that installation of a traffic signal at the intersection of Northwest Highway and Tulane Boulevard will NOT be warranted at full buildout of the potential PD 15 redevelopment. The findings of this analysis are subject to the review by and approval of the City of Dallas staff.

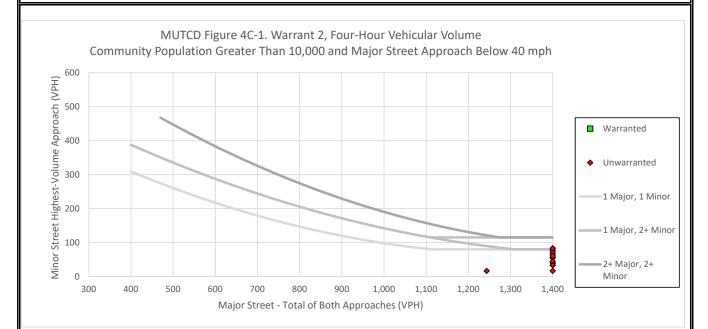
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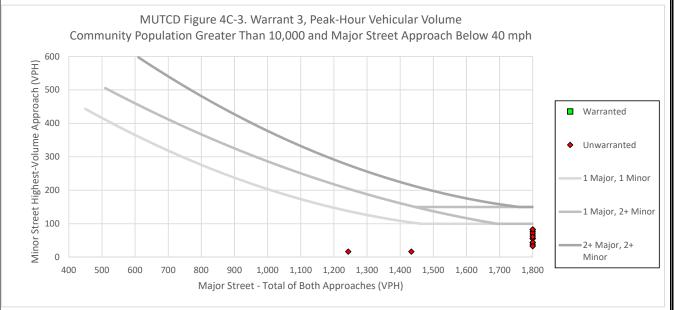


**APPENDIX** 

Date:5/20/2019Intersection:Tulane Boulevard at Northwest HighwayConditions:Projected VolumesProject #:3602-17.341







	Date:				5	5/20/2019				1/1	Pachec	Koch			
Intersection: Tu				ane Boulevard	at Northwes	t Highway	-								
Conditions:					Projected	d Volumes	-	Community Population < 10,000?							
Project #:					36	02-17.341	-		Speed Limit	> 40 mph?					
							-								
	<u>Lanes</u> <u>Lanes</u>						,	Warrant							
		Major Street 3					8-H		1						
		lane Boulev			thwest High		(100%)	(100%)	, -	0%)	1				
	EB	WB	TOTAL	NB		MAX		Condition B	Condition A	Condition B	4-Hour	Peak-Hour			
6:00 AM	744	,		0		66		Not Met	Not Met	Not Met	Not Met	Not Met			
7:00 AM	1,538	3,215		0		83		Not Met	Not Met	Met	Not Met	Not Met			
8:00 AM	1,650	,		0		55		Not Met	Not Met	Not Met	Not Met	Not Met			
9:00 AM	1,464			0		77		Not Met	Not Met	Not Met	Not Met	Not Met			
10:00 AM	1,451	1,269		0		39		Not Met	Not Met	Not Met	Not Met	Not Met			
11:00 AM	1,777	1,131	2908	0		33		Not Met	Not Met	Not Met	Not Met	Not Met			
12:00 PM	1,355	1,022	2377	0	72	72	Not Met	Not Met	Not Met	Not Met	Not Met	Not Met			
1:00 PM	1,019	869	1888	0	55	55	Not Met	Not Met	Not Met	Not Met	Not Met	Not Met			
2:00 PM	1,193	1,122	2314	0	55	55	Not Met	Not Met	Not Met	Not Met	Not Met	Not Met			
3:00 PM	1,499	1,259	2758	0	83	83	Not Met	Not Met	Not Met	Met	Not Met	Not Met			
4:00 PM	2,533	2,008	4542	0	61	61	Not Met	Not Met	Not Met	Not Met	Not Met	Not Met			
5:00 PM	2,493	1,988	4481	0	44	44	Not Met	Not Met	Not Met	Not Met	Not Met	Not Met			
6:00 PM	1,877	877	2754	0	44	44	Not Met	Not Met	Not Met	Not Met	Not Met	Not Met			
7:00 PM	1,278	557	1835	0	33	33	Not Met	Not Met	Not Met	Not Met	Not Met	Not Met			
8:00 PM	932	502	1434	0	17	17	Not Met	Not Met	Not Met	Not Met	Not Met	Not Met			
9:00 PM	785	458	1243	0	17	17	Not Met	Not Met	Not Met	Not Met	Not Met	Not Met			
	•	•	'	1							•	•			
							Required	l Volume		# Hours	# Hours				
							Major	Minor		Satisfied	Required	Warrant			
1. Eight-Ho	our Warrant										•				
			tion A Met?				600	200		0	8				
	(100%)		tion B Met?				900	100		0					
	(0.5-1)	Condtion A Met?				480				8					
	(80%)		tion B Met?				720				8				
2. Four-Ho	ur Warrant								1		_				
			Met?							0	4	Not Met			
3. Peak-Ho	our Warrant										<u> </u>				
			Met?							0	1	Not Met			

(based upon ITE Trip Generation handbook, 10th Edition)

Existing				Daily	<u>AM</u>	<u>In</u>	Out	<u>PM</u>	<u>In</u>	Out	
	High-Rise		460 DU	2024	142	34	108	165	101	64	
	Low-Rise		74 DU	519	36	8	28	45	28	17	
				2543	177	42	135	210	129	81	_
<u>Scenario</u>	1 - Entire acreage gets redeveloped		12.4 acres	<u>Daily</u>	<u>AM</u>	<u>ln</u>	<u>Out</u>	<u>PM</u>	<u>ln</u>	<u>Out</u>	
A) @	90 DU/acre	-	Total	5247	371	89	282	443	270	173	
.,, C	55 - 5,555			206%	209%	211%	208%	211%	209%	213%	inc./existing
B) @	125 DU/acre		1550 DU	7205	510	122	387	612	373	239	
ы ш	123 DO/acre		1330 00	283%	287%	290%	286%	291%	289%	295%	inc./existing
Scenairo	2 - Only existing low-rise gets redevel	oped_	6 acres	<u>Daily</u>	<u>AM</u>	<u>In</u>	<u>Out</u>	<u>PM</u>	<u>In</u>	<u>Out</u>	
A) @	90 DU/acre	New _	Total 540	2339	164	39	125	192	117	75	
-,, -,		Existing	460	2024	142	34	108	165	101	64	
			1000 DU	4363	306	73	233	357	218	139	_
	DIFFERENCE (OVER EXISTING)		1000 DU	4363 1820	306 128	73 <b>31</b>	233 97	357 147	218 <b>89</b>	139 <b>58</b>	_
	DIFFERENCE (OVER EXISTING)		1000 DU								inc./existing
В) @	DIFFERENCE (OVER EXISTING)  125 DU/acre	New	1000 DU	1820	128	31	97	147	89	58	inc./existing
В) @	,	New Existing		1820 172%	128 172%	<b>31</b> 173%	9 <b>7</b> 172%	147 170%	89 169%	<b>58</b> 172%	inc./existing
В) @	,		750	1820 172% 3167	128 172% 223	31 173% 53	97 172% 170	147 170% 264	89 169% 161	58 172% 103	inc./existing
В) @	,		<b>750</b> 460	1820 172% 3167 2024	128 172% 223 142	31 173% 53 34	97 172% 170 108	147 170% 264 165	89 169% 161 101	58 172% 103 64	inc./existing

